ME 478 FINITE ELEMENT METHOD

Chapter 8. Other Cool Stuff

LINEAR ELASTO-DYNAMICS

$$\mathbf{M}\ddot{\mathbf{u}} + \mathbf{K}\mathbf{u} = \mathbf{f}_{\varepsilon_{a}} + \mathbf{f}_{b} + \mathbf{f}_{t} + \mathbf{P}$$

where

$$u(x, y, t) = \mathbf{Nu}$$

$$\mathcal{E}(x, y, t) = \mathbf{Bu}$$

$$\sigma(x, y, t) = \mathbf{D}(\mathbf{Bu} - \boldsymbol{\varepsilon}_{o})$$

and mass matrices are:

and the stiffness and mass matrices are:

$$\mathbf{K} = \iiint_{V} \mathbf{B}^{T} \mathbf{D} \mathbf{B} dv \qquad \mathbf{M} = \iiint_{V} \mathbf{N}^{T} \rho \mathbf{N} dv$$

Note that **M** and **K** have the same form (due to connectivity)

MODAL ANALYSIS

We will start by looking at the free vibration problem

 $\mathbf{M}\ddot{\mathbf{u}} + \mathbf{K}\mathbf{u} = 0 \qquad (\mathbf{f} = 0)$

let $u(t) = \hat{\mathbf{u}} \sin(\omega t + \theta)$ where $\hat{\mathbf{u}}$ are the mode shapes, then by substitution, we have:

$$(\mathbf{K} - \boldsymbol{\omega}^2 \mathbf{M})\hat{\mathbf{u}} = 0$$

for a nontrivial solution to exist, we require

$$\det(\mathbf{K} - \boldsymbol{\omega}^2 \mathbf{M}) = 0$$

from which we can solve for the frequencies and then the corresponding mode shapes.

EXAMPLE: 1-D BAR Recall that we had:

$$u(s) = \begin{bmatrix} N_1 & N_2 \end{bmatrix} \begin{bmatrix} u_1 \\ u_2 \end{bmatrix}$$

where

$$N_1 = \frac{1-s}{2}$$
 $N_2 = \frac{1+s}{2}$

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and the strains are related to the displacements through:

$$\mathbf{\varepsilon}_{s} = \begin{bmatrix} \frac{-1}{L} & \frac{1}{L} \end{bmatrix} \begin{bmatrix} u_{1} \\ u_{2} \end{bmatrix} = \mathbf{B}\mathbf{u}$$

And the stress/strain relation as:

$$\sigma_s = EBu$$

We apply Newton's second law of motion to the 2 nodes F = ma

$$f_{1ext} - f_{1int} = m_1 \frac{\partial^2 u_1}{\partial t^2}$$
$$f_{2ext} - f_{2int} = m_2 \frac{\partial^2 u_2}{\partial t^2}$$

where $m_1 = \rho \frac{AL}{2}$ and $m_1 = \rho \frac{AL}{2}$ (called the lumped mass) Writing out the equations, we have:

$$\begin{cases} f_{1ext} \\ f_{2ext} \end{cases} = \begin{cases} f_{1int} \\ f_{2int} \end{cases} + \begin{bmatrix} m_1 & 0 \\ 0 & m_2 \end{bmatrix} \begin{cases} \ddot{u}_1 \\ \ddot{u}_2 \end{cases}$$

Consistent Mass matrix Starting with d'Alembert's principle

$$\mathbf{X}^e = -\rho \ddot{\mathbf{u}}(x, y)$$

$$\mathbf{f}_b = \iiint_V \mathbf{N}^T \mathbf{X}^e \, dv$$

Making the substitution for \mathbf{X}^e and knowing that $\ddot{\mathbf{u}}(x, y) = \mathbf{N}\ddot{\mathbf{u}}$, we have

$$\mathbf{f}_b = \iiint_V \mathbf{N}^T \boldsymbol{\rho} \mathbf{N} dv \mathbf{\ddot{u}}$$

ELASTODYNAMICS INCLUDING INELASTICITY Starting with:

$$\mathbf{M}\ddot{\mathbf{u}} + \mathbf{K}\mathbf{u} = \mathbf{f}$$

where

$$\mathbf{s} = \iiint_V \mathbf{B}^T \boldsymbol{\sigma} \, dv$$

Now including rate effects $\sigma = D\epsilon + V\dot{\epsilon}$

$$\mathbf{s} = \iiint_V \mathbf{B}^T \mathbf{D} \mathbf{B} \, dv \mathbf{u} + \iiint_V \mathbf{B}^T \mathbf{V} \mathbf{B} \, dv \dot{\mathbf{u}}$$

So we have

$$\mathbf{M}\ddot{\mathbf{u}} + \mathbf{C}\dot{\mathbf{u}} + \mathbf{K}\mathbf{u} = \mathbf{f}$$

SPECIAL CASES

1) No damping-Linear elasto-dynamics

 $M\ddot{u} + Ku = f$

2) No inertial effects-elasto-statics

$\mathbf{K}\mathbf{u} = \mathbf{f}$

3) Quasi-static-Visco-elasto statics

$\mathbf{C}\mathbf{\dot{u}} + \mathbf{K}\mathbf{u} = \mathbf{f}$

4) Viscous flow (Newtonian fluids)

$\mathbf{C}\mathbf{\dot{u}} = \mathbf{f}$

RHEOLOGICAL MODELS









Parallel Arrangement (Voigt-Kelvin Model)



During the creep response, the material represented by these models undergoes

- 1. Initial instantaneous response
- 2. Non-linear delayed elastic response
- 3. Instantaneous elastic recovery
- 4. Delayed elastic-viscoelastic recovery
- 5. Permanent deformation

For the Maxwell model, the relationship between the deflection and the applied load is:

$$\dot{u} = \frac{\dot{P}}{k} + \frac{P}{\eta}, \qquad u(0) = \frac{P(0)}{k}$$

The resulting creep function for a unit step is:

$$c(t) = \left(\frac{1}{k} + \frac{1}{\eta}t\right) U(t)$$

and the relaxation function is:

$$r(t) = k e^{-(k/\eta)t} U(t)$$

For the Voigt model (slightly different than above), the relationship between the deflection and the applied load is:

$$P = ku + \eta \dot{u}, \quad u(0) = 0$$

The resulting creep function for a unit step is:

$$c(t) = \frac{1}{k} \left(1 - e^{-(k/\eta)t} \right) U(t)$$

and the relaxation function is:

$$r(t) = \eta \delta(t) + kU(t)$$

Viscoelastic Overstress – Delayed Elasticity

This element is associated with some threshold condition





And our stress-strain relation becomes: $\dot{\sigma} = \mathbf{D} : (\dot{\epsilon} - \dot{\epsilon}_p)$ $\overbrace{E_{\varepsilon_p} \\ \varepsilon_e}^{\sigma}$ ELASTO-VISCO-PLASTICITY $\overbrace{E_{\varepsilon_p} \\ \varepsilon_e}^{\sigma}$ $E_T = \frac{\partial \sigma}{\partial \varepsilon} = \frac{E E_p}{E + E_p}$ $\overbrace{E_{\varepsilon_p} \\ \varepsilon_e}^{\sigma}$ $E_p = \frac{\partial \sigma}{\partial \varepsilon_p}$

This is a combination of the three rheological models. Solution procedure is much more complicated than for linear elasticity

Example One Dimensional Viscoplasticity

Consider the basic one-dimensional viscoplastic model. The total strain n the model can be expressed as the sum of the elastic and the viscoelstic components as:

$$\varepsilon = \varepsilon_e + \varepsilon_{vp}$$

The applied stress is related to the elastic strain by:

$$\sigma_e = \sigma = E \varepsilon_e$$

The stress in the dashpot is related to the viscoplastic strain by:

$$\sigma_d = \eta \dot{\epsilon}_{vp}$$

And the stress in the friction slider is:

$$\sigma_{p} = \sigma \quad if \quad \sigma_{p} < Y$$
$$\sigma_{p} = Y \quad if \quad \sigma_{p} \ge Y$$

where Y represents the threshold stress which is a function of some yield stress and some strain hardening as:

$$Y = \sigma_y + H' \varepsilon_{vp}$$

Prior to the onset of viscoplastic yielding, $\mathbf{\epsilon}_{vp} = 0$ giving $\sigma_d = 0$

thus $\sigma_p = \sigma$. Combining stresses in the dashpot and the friction slider gives:

$$\sigma = \sigma_{y} + H' \varepsilon_{vp} + \eta \dot{\varepsilon}_{vp}$$

Using $\varepsilon_{vp} = \varepsilon - \varepsilon_e$ and $\sigma_e = \sigma = E \varepsilon_e$ gives

$$H' E\varepsilon + \eta E\dot{\varepsilon} = H' \sigma + E(\sigma - \sigma_y) + \eta \dot{\sigma}$$

which is a first order ODE, Rearranging, we get



$$\dot{\varepsilon} = \frac{\dot{\sigma}}{E} + \frac{1}{\eta} [\sigma - (\sigma_y + H' \varepsilon_{vp})]$$

or

 $\dot{\varepsilon} = \dot{\varepsilon}_e + \dot{\varepsilon}_{vp}$

Considering the case when we apply a constant stress to the model

$$H' E\varepsilon + \eta E\dot{\varepsilon} = H' \sigma_A + E(\sigma_A - \sigma_y)$$

The solution is:

$$\varepsilon(t) = \frac{\sigma_A}{E} + \frac{(\sigma_A - \sigma_y)}{H'} [1 - e^{-H't/\eta}]$$

Note: the solution to ODE's of the form y'+p y = r is

$$y(t) = e^{-h} \int e^{h} r dt + C$$
 where: $h = \int p dt$

In the case of a perfectly viscoplastic material, (H'=0) then we have (by applying L'Hopital's rule):

$$\varepsilon(t) = \frac{\sigma_A}{E} + \frac{(\sigma_A - \sigma_y)t}{\eta}$$

Viscoplasticity is a transient phenomena, thus the solution involves taking a time incremental (time-stepping) approach. The simplest approach is to use Euler's rule where we extrapolate the value at some time t_{n+1} in terms of the quantities at time t.

Using this approach, we can define the viscoplastic strain

increment over time step $\Delta t_n = t_{n+1} - t_n$ as:

$$\Delta \mathcal{E}_{vp}^{n} = \dot{\mathcal{E}}_{vp}^{n} \Delta t_{n}$$

The change in length of the element due to the strain increment is:

$$\Delta u^n = \Delta \mathcal{E}_{vp}^n L$$

and adding this to the change in length due to the applied loading gives:

$$\Delta u^n = \Delta \mathcal{E}_{vp}^n L + \frac{L}{AE} \Delta P_n$$

and rewriting in matrix form gives:

$$\Delta u^{n} = \begin{cases} \Delta u_{1}^{n} \\ \Delta u_{2}^{n} \end{cases} = \mathbf{K}^{-1} \Delta F_{n}$$

Where

$$\Delta F_n = AE\dot{\varepsilon}_{vp}^n \Delta t_n \begin{cases} 1\\ -1 \end{cases} + \Delta P_n$$

and

$$\mathbf{K} = \frac{EA}{L} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix}$$

Rewritting in the standard form

$$\mathbf{K} = \int_{V} \mathbf{B}^{T} \mathbf{D} \mathbf{B} dv$$
$$\Delta F_{n} = \int_{V} \mathbf{B}^{T} \mathbf{D} \varepsilon dv + \Delta P_{n}$$

The updated displacements at obtained as:

$$u^{n+1} = u^n + \Delta u^n$$

The stress increment is:

$$\Delta \sigma^{n} = E \Delta \varepsilon_{e}^{n} = E(\Delta \varepsilon^{n} - \Delta \varepsilon_{vp}^{n})$$
$$= E \left(\frac{\Delta u_{1}^{n} - \Delta u_{2}^{n}}{L} - \dot{\varepsilon}_{vp}^{n} \Delta t_{n} \right)$$

The stress at time t_{n+1} is:

$$\sigma^{n+1} = \sigma^n + \Delta \sigma^n$$

The viscoplastic strain is:

$$\boldsymbol{\varepsilon}_{vp}^{n+1} = \boldsymbol{\varepsilon}_{vp}^{n} + \Delta \boldsymbol{\varepsilon}_{vp}^{n}$$

And lastly, the viscoplastic strain rate is:

$$\dot{\varepsilon}_{vp}^{n+1} = \frac{1}{\eta} [\sigma^{n+1} - (\sigma_y + H' \varepsilon_{vp}^{n+1})]$$

The out of balance residual forces as expressed as the sum of the applied nodal loads and the nodal forces equivalent to the elemental stresses are:

$$\Psi_{n+1} = A \sigma^{n+1} \begin{cases} 1 \\ -1 \end{cases} + P_{n+1}$$

These residual forces are added to the pseudo forces to give the forces for the next time increment as:

$$\Delta F_{n+1} + = AE\dot{\varepsilon}_{vp}^{n+1}\Delta t_{n+1} \begin{cases} 1\\ -1 \end{cases} + \Delta P_{n+1} + \Psi_{n+1}$$

This step is repeated until the solution for the desired time duration or steady state conditions are achieved (when the viscoplastic strain rate becomes negligible).

There is a limit on the time step that one can use for the viscoplastic solution. For the one-dimensional case considered here, the limiting value is (Cormeau proposed this one and there are many different values that have been proposed):

$$\Delta t \leq \frac{\eta \sigma_y}{E}$$

Computational Implementation of 1-D Viscoplasticity

STEP 1 At time $t=t_n$, we compute that following quantities using the standard approach

 σ^n , ε^n , ε^n , f^n , and u^n known

and compute the viscoplastic strain rate for each element as:

$$\dot{\varepsilon}_{vp}^{n} = \frac{1}{\eta} [\sigma^{n} - (\sigma_{y} + H' \varepsilon_{vp}^{n})]$$

STEP 2 Compute the displacement increment according to:

$$\Delta u^{n} = \begin{cases} \Delta u_{1}^{n} \\ \Delta u_{2}^{n} \end{cases} = \mathbf{K}^{-1} \Delta F_{n}$$

where

$$\Delta F_n = AE\dot{\varepsilon}_{vp}^n \Delta t_n \begin{cases} 1\\ -1 \end{cases} + \Delta P_n$$

and for each element, the stiffness matrix is

$$\mathbf{K} = \frac{EA}{L} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix}$$

STEP 3 Compute the stress increment and the viscoplastic strain increment for each element as:

$$\Delta \sigma^{n} = E \left(\frac{\Delta u_{1}^{n} - \Delta u_{2}^{n}}{L} - \dot{\varepsilon}_{vp}^{n} \Delta t_{n} \right)$$
$$\Delta \varepsilon_{vp}^{n} = \dot{\varepsilon}_{vp}^{n} \Delta t_{n}$$

STEP 4 Determine the total displacements, stresses and the viscoplastic strains

$$u^{n+1} = u^{n} + \Delta u^{n}$$
$$\sigma^{n+1} = \sigma^{n} + \Delta \sigma^{n}$$
$$\varepsilon_{vp}^{n+1} = \varepsilon_{vp}^{n} + \Delta \varepsilon_{vp}^{n}$$

STEP 5 Compute the Viscoplastic strain rate for each element for the next time step

$$\dot{\varepsilon}_{vp}^{n+1} = \frac{1}{\eta} [\sigma^{n+1} - (\sigma_{y} + H' \varepsilon_{vp}^{n+1})]$$

STEP 6 Evaluate the residual forces by applying equilibrium correction to each element

$$\Psi_{n+1} = A\sigma^{n+1} \begin{cases} 1 \\ -1 \end{cases} + \Delta P_{n+1}$$

and add these into the vector of pseudo nodal loads to be used in the next time step

$$\Delta F_{n+1} = AE\dot{\varepsilon}_{vp}^{n+1}\Delta t_{n+1} \begin{cases} 1\\ -1 \end{cases} + \Delta P_{n+1} + \Psi_{n+1}$$

STEP 7 Check if the viscoplastic strain rate in each element has become negligibly small. If so, then steady state conditions are said to have been reached. If not, return to STEP 1 and repeat the entire process for the next time step. One way to check for convergence is as follows:

$$\frac{\sum_{i=1}^{M} \left| \left(\Delta \varepsilon_{vp}^{n} \right)_{i} \right|}{\sum_{i=1}^{M} \left| \left(\Delta \varepsilon_{vp}^{1} \right)_{i} \right|} \times 100 \leq TOLERANCE$$